

Demonstration Plant

January 11, 2012

Abstract

The AguaClara program needs appropriate and accurate teaching and publicity aids. The team developed a demonstration plant several years ago that illustrated the early flow control module, the baffled flocculator, and a sedimentation tank with plate settlers. In the intervening years we have developed a dose controller that tracks the plant flow rate, sedimentation tanks designed to include floc blankets, and stacked rapid sand filters. The next generation of the AguaClara demonstration unit should illustrate as many of these concepts as possible.

The demonstration plant will be used as a portable demonstration unit to advertise the AguaClara technologies. The AguaClara DP will be a centerpiece of the EPA P3 competition in April of 2012.

Students 8

Skills fluids, AguaClara water treatment processes, process controller, fabrication

Location Project Lab

1 Introduction

The AguaClara DP will be an excellent educational tool and demonstration unit as well as a device that can be used in households. It will be used at Cornell for outreach activities as well as by implementation partners as they promote the AguaClara technologies to municipalities.

This team can begin learning the complexities of small scale water treatment by setting up a bench scale water treatment plant using the ENGRI 1131 apparatus. As the team is setting up the bench scale model they can also begin designing components of a new demonstration plant, DP, that incorporates as many of the AguaClara concepts as are feasible. The DP flow rate should be as low as possible to reduce the need for large storage tanks for the raw water. The minimum flow rate may be set by the sedimentation tank or by the stacked rapid sand filter, SRSF.

This team will need to divide into 4 sub teams to tackle each component of the AguaClara plant and will need a strong effective leader who can coordinate all of the sub teams. The demo plant must be ready for prime time to help us win the EPA P3 competition in April of 2012

1. Flow Control, coagulant dosing, and flocculator
2. Sedimentation, floc blanket, floc hopper, plate settlers and floc recycle to the flocculator
3. SRSF including inlet and outlet control boxes and siphon control system
4. Integration, table top mounting system (using 80/20), aesthetics, knock down guidelines for transport, etc.

Each of the three students on Team 4 will work on one of the the other teams throughout the semester to facilitate collaboration between teams and include transport and aesthetic considerations into the design process from the beginning.

2 Design Strategy

Create a detailed Mathcad worksheet with design equations for each unit process. Create equations for all relevant dimensions. Many of the design equations can be borrowed from the AguaClara design tool files. However, care must be taken for flocculator design to account for the laminar flow. The design tool files assume turbulent flow for the flocculator. The available pipe sizes will also need to be modified because the pipe database doesn't include the small diameter tubes that would be used in the DP.

2.1 Flow measurement and dose control

Explore the possibility of using a linear flow orifice meter, LFOM, to measure the raw water flow into the DP. It is possible that the required orifice size for an LFOM would be too small to be practical. The LFOM created in the fall semester appears to perform poorly and thus a better flow measurement system is required (see Fall 2011 DP Final Report). Given the problems with the LFOM it is likely necessary to switch to a long laminar flow tube (or several tubes) to generate a linear relationship between flow through the DP and elevation of water in the DP entrance tank. Explore the possibility of using a chemical dose controller for the coagulant (polyaluminum chloride, PACl, or aluminum sulfate, alum). It would be excellent if this demonstration plant could fully illustrate how a chemical dose controller works. The dose controller will require significant elevation. Full scale plants use 20 cm of elevation change in the entrance tank. Perhaps the elevation change in the entrance tank could be reduced slightly. However, if the elevation is reduced too much surface tension will cause large errors in dosing.

2.2 Tube Flocculator

Design a tube flocculator to be as short as possible given the maximum flow rate predicted above. The velocity gradient, G , will likely be in the range of 30 to 100/s. Explore the relationship between G and maximum floc size to determine the optimal value for G .

$$\bar{G} = \frac{64Q}{3\pi D^3} = \frac{16V}{3D} \quad (1)$$

The relationship between velocity gradient and energy dissipation rate for laminar flow is

$$\bar{\epsilon} = \bar{G}^2 \nu \quad (2)$$

Thus the average energy dissipation rate can be calculated from the velocity and diameter of the tube flocculator. Note that this approach is neglecting the additional velocity gradients caused by using coiled tubing.

$$\bar{\epsilon} = \left(\frac{16V}{3D}\right)^2 \nu \quad (3)$$

The maximum velocity gradient occurs at the wall of the tube.

$$G_0 = 8\frac{\bar{V}}{D} \quad (4)$$

The corresponding maximum energy dissipation rate for a tube flocculator is thus

$$\epsilon_{Max} = 64\left(\frac{\bar{V}}{D}\right)^2 \nu \quad (5)$$

The ratio of maximum to average energy dissipation rate for laminar flow is thus

$$\alpha_\epsilon = \frac{\epsilon_{Max}}{\bar{\epsilon}} = \frac{9}{4} \quad (6)$$

The AguaClara designs currently use $10 \frac{mW}{kg}$ for the maximum energy dissipation rate. The corresponding \bar{G} is

$$\bar{G} = \frac{2}{3} \sqrt{\frac{10 \frac{mW}{kg}}{1 \frac{mm^2}{s}}} = 67 \text{ }^1\text{/s} \quad (7)$$

The required diameter of a laminar flow flocculator given a flow rate and a target maximum energy dissipation rate can be obtained by combining equation (5) with the continuity equation.

$$\epsilon_{Max} = 64 \left(\frac{4Q}{D^3\pi} \right)^2 \nu \quad (8)$$

$$D = \left(\frac{32Q}{\pi} \right)^{\frac{1}{3}} \left(\frac{\nu}{\epsilon_{Max}} \right)^{\frac{1}{6}} \quad (9)$$

The equations above do not account for the coiling of the flocculator tube. The Dean number, Π_{De} is used to characterize coiled tubing.

$$\Pi_{De} = \frac{VD}{\nu} \left(\frac{D}{2D_{Coil}} \right)^{\frac{1}{2}} \quad (10)$$

Liu and Masliyah's model (1993) for Dean numbers less than 5000 gives the ratio of the friction factor of curved versus straight tubing.

$$f_{ratio} = \frac{1 + \left[0.0908 + 0.0233 \left(\frac{D}{D_{Coil}} \right)^{\frac{1}{2}} \right] \Pi_{De}^{\frac{1}{2}} - 0.132 \left(\frac{D}{D_{Coil}} \right)^{\frac{1}{2}} + 0.37 \left(\frac{D}{D_{Coil}} \right) - 0.2}{1 + \frac{49}{\Pi_{De}}} \quad (11)$$

Head loss in the tubing is proportional to the friction factor, f , and the energy dissipation rate is proportional to the head loss. The energy dissipation rate for a coiled tube is equal to the energy dissipation rate for a straight tube scaled by the f_{ratio} .

The collision potential for a laminar flow flocculator is proportional to the product of the velocity gradient and the residence time, θ . The collision potential is a measure of the ability of the flocculator to cause flocs to collide.

$$G\theta = \frac{16L}{3D} \quad (12)$$

The fractal flocculation model predicts that for 50 NTU water that $G\theta$ of 1400 would be adequate to produce $70 \mu m$ flocs and those flocs should be able to settle out with a capture velocity of 0.12 mm/s. Thus for demonstration purposes where we would generally use very turbid water it may be adequate to use a relatively short residence time. Equation 12 can be solved for the length of the flocculator.

2.3 Sedimentation Tank

The sedimentation tank should include a floc blanket, floc weir, floc hopper, and plate settlers. Use the designs from the ENGRI 1131 competition as starting points. The plate settler will likely be replaced with a simple tube settler. The floc blanket will require a section of the reactor that has vertical walls and an up flow velocity of 1 to 2 $\frac{mm}{s}$.

The relationship between the jet energy dissipation rate and the diameter of the pipe that discharges the flocculator water into the sedimentation tank is highly dependent on the inlet geometry. For the

case where the pipe discharges upward and there is no direction change (no vena contracta) as the fluid exits the pipe the equation is

$$D_{Pipe} = \left(\frac{Q_{Pipe} 4\Pi_{Jet}}{\epsilon_{Max}^{\frac{1}{3}} \pi} \right)^{\frac{3}{7}} \quad (13)$$

where Q_{Pipe} is the volumetric flow rate in the pipe, ϵ_{Max} is the maximum energy dissipation rate produced by the jet, and Π_{Jet} has a value of approximately 0.5. The relationship between average energy dissipation rate, $\bar{\epsilon}$, and maximum energy dissipation rate in a jet is

$$\epsilon_{Max} = \alpha_{\epsilon} \bar{\epsilon} \quad (14)$$

where α_{ϵ} is the ratio of maximum to average energy dissipation rate and has a value of about 2 for jets. The maximum floc size was measured by Ian Tse with a tube flocculator.

$$D_{Floc} = 75 \mu m \left(\frac{\bar{\epsilon}}{\frac{W}{kg}} \right)^{-\frac{1}{3}} \quad (15)$$

It should be possible to combine equations 13 to 15 to solve for the required pipe diameter given a target floc diameter. Ideally the flocs would have a sedimentation velocity that matches the up flow velocity in the floc blanket to ensure that they aren't carried up and out of the sedimentation tank. Or perhaps they only need to have a sedimentation velocity that matches the tube settler capture velocity. The previous attempts at producing a floc blanket with 2.5 cm diameter sedimentation tanks did not include tube settlers and thus it is possible that this floc breakup problem will not occur if tube settlers are in place to return flocs that they capture. Floc sedimentation velocity is given by

$$V_t = \frac{gd_0^2}{18\Phi\nu_{H_2O}} \frac{\rho_{Floc0} - \rho_{H_2O}}{\rho_{H_2O}} \left(\frac{d}{d_0} \right)^{D_{Fractal}-1} \quad (16)$$

where d_0 is the diameter of the primary particles, d is the floc diameter, Φ is a fluid drag correction factor for the non spherical flocs, ν_{H_2O} is the kinematic viscosity of water, ρ_{Floc0} is the density of the primary particles, ρ_{H_2O} is the density of water, and $D_{Fractal}$ is the fractal dimension of the flocs. The graph of equation 16 is shown in Figure 1. The flocs in the floc blanket may grow in size to be several mm in diameter and thus have very high sedimentation velocities. The up flow velocity in the floc blanket is 1 to 2 mm/s and the capture velocity in AguaClara plate settlers is set to 0.12 mm/s although it may be increased significantly for the demo plant.

The floc weir will set the maximum level of the floc blanket. The floc blanket should probably have a depth between 30 and 60 cm. Floc blanket particle capture efficiency improves with depth, but the goal for the demonstration plant is to keep the unit processes small and to sacrifice performance if necessary. The floc hopper will likely have a plan view area that is 10% of the plan view area of the floc blanket.

The inlet conditions for the flocculated water in the sedimentation tank need to be very carefully considered. The small diameter of the jet causes the energy dissipation rate to be high and that may cause excessive floc breakup. The optimal configuration is a jet that enters through the bottom of the sedimentation tank so that no jet reverser is required. The diameter of the inlet should be set to ensure that the maximum energy dissipation rate, ϵ_{Max} , of the resulting jet is less than $10 \frac{mW}{kg}$.

$$\epsilon_{Max} = \frac{(\Pi_{Jet} V_{Jet})^3}{D_{Jet}} \quad (17)$$

where Π_{Jet} has a value of 0.4. Note that in the case of a tube discharging upward into the sedimentation tank that there is no vena contracta. The jet must be released at the bottom of a cone that collects all settled flocs and directs them toward the jet for resuspension.

We will be testing the use of floc recycle to improve flocculator performance this semester and it is likely that we will want to incorporate floc recycle into the demonstration plant.

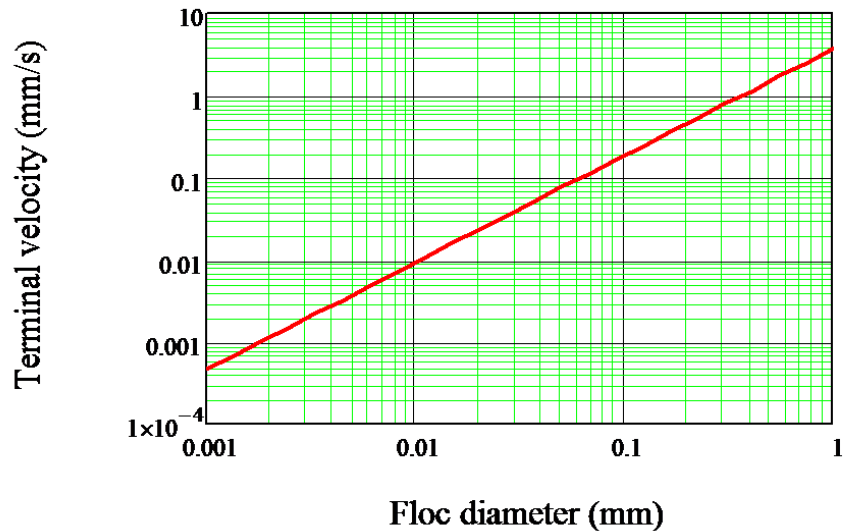


Figure 1: Floc sedimentation velocity as a function of size.

2.4 Stacked Rapid Sand Filter

The stacked rapid sand filter is a hydraulically complex system that would be incredibly useful to be able to demonstrate at bench scale. A full scale unit has a depth of about 3.7 m based on 6 20 cm filter layers and then the elevation required for the fluidized bed and backwash head loss. This will require scaling the depth of the filter layers down significantly to make a bench top model. One possibility will be to reduce the number of layers to 4 and reduce the filter layer depth to approximately 4 cm. The plumbing dimensions will also have to be carefully considered. The vertical drop tubes on the inlet side of the filter need to be large enough to allow counter current flow of air and water. That constraint requires a tubing ID of about 9 mm. It would also be beneficial if the vertical drop tubes had a small angle away from the vertical to making it easier for water to flow down the bottom of the tube and air to flow up on the top side of the tube.

The inlet and outlet manifolds could be custom manufactured stainless steel tubes with tiny slots. We have the ability to cut the slots using a slotting saw that is similar to how the slotted PVC pipes are created. As many components as possible should be transparent to facilitate direct observation of the hydraulics.

The flow rate for the SRSF needs to be coordinated with the other processes. Similarly, the controls for the SRSF including the inlet and outlet boxes and weirs need to be designed as an integral part of the filter system. It may be advantageous to use a smaller diameter sand. However, we should first try to use the 0.5 mm diameter sand to see how that works. Going to smaller sand will increase the risk of sand entering the slotted pipes. The backwash and filtration velocities will need to be reduced if the sand diameter is reduced. Use the Stacked Rapid Sand Filter Mathcad design file as a basis to design the filter system. Use the same logic to size the piping components to ensure that there is reasonable flow distribution between layers.

3 General Considerations

The demo plant should be easy to operate, easy to assemble and transportable as a carry-on luggage item. The unit processes should be easy to disconnect and clean. The plumbing connections must all be leak tight to prevent spills. The water level in the plant must be controlled with an exit weir from

the filter.

The various unit processes can be mounted on a central 80/20 tower. Flexible tubing could be used to connect the unit processes. The SRSF and possibly the sed tank could hang off of the edge of the table to keep the overall height reasonable.

The flow rate for the demo plant is directly related to the cross sectional area of the unit processes. If we use AguaClara design guidelines, then the filter backwash velocity is $11 \frac{mm}{s}$ and the upflow velocity in the floc blanket is $1 \frac{mm}{s}$ or perhaps $2 \frac{mm}{s}$. That means that the ratio of the diameters of these unit process is $\sqrt{11}$ or $\sqrt{5.5} = 2.3$. The high flow rate of the filter suggests that it would be useful to make the filter diameter as small as possible. One possible set of diameters would be to set the SRSF diameter to 1 cm and the sed tank to 2.5 cm.

References

- [1] Liu, S. and Masliyah, J.H. (1993). Axially Invariant Laminar Flow in Helical Pipes with a Finite Pitch. *J. Fluid Mech.* 251, 315-353.